



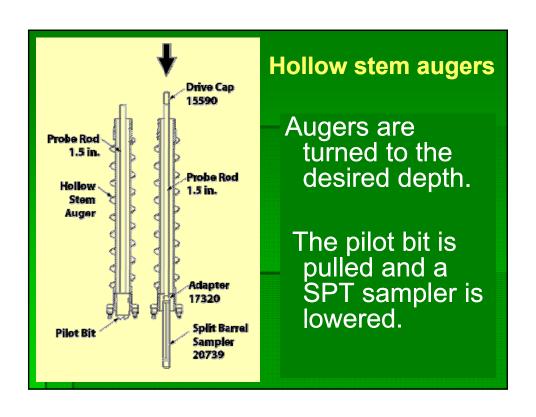


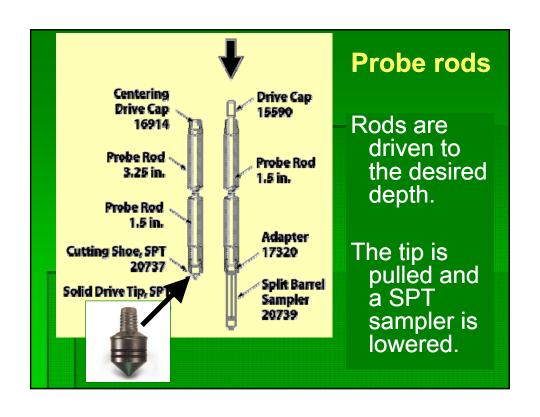


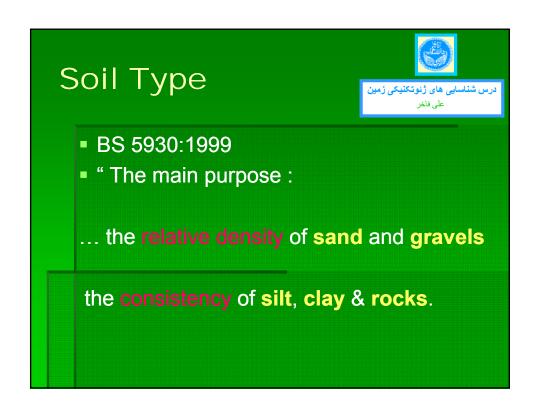


# روشهای جایگزین حفر گمانه برای انجام این آزمایش

- Hollow stem augers.
- Probe rods.







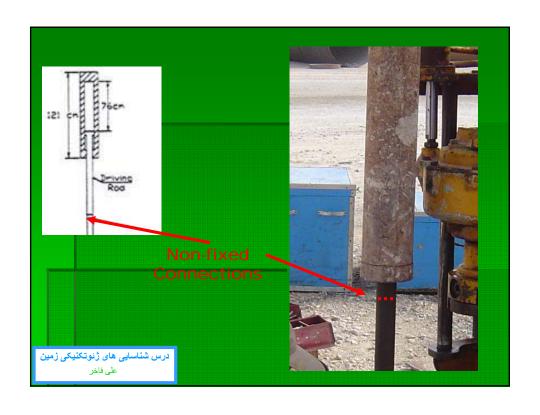












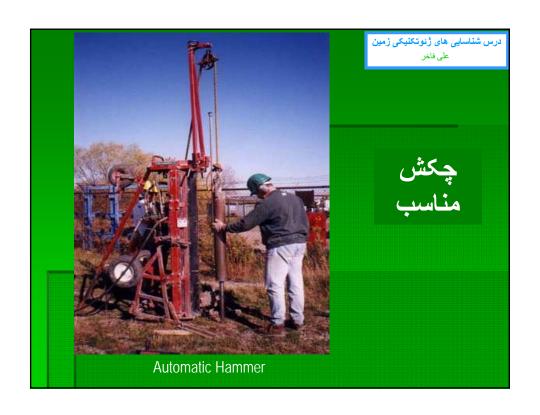


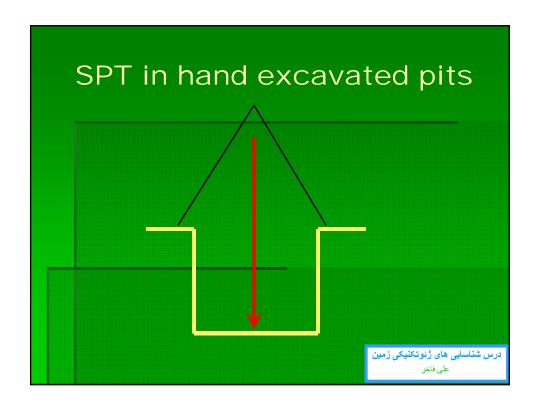












# Due to the effect of borehole diameter آزمایش انجام شده در کف چاهک استاندارد نیست پرس شنسلی های ژبوتکنیکی زمین

## لزوم روابط تجربى محلى

- BS 5930:1999
- SPT results and soil parameters derived from data from other countries may not correlate with results ...in accordance with BS 1377-9.
- So local correlations are required

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# Energy measurements are required in Iran

- For different hammer &
- different combinations of drill rig,

hammer,

rods &

drillers

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### Example:

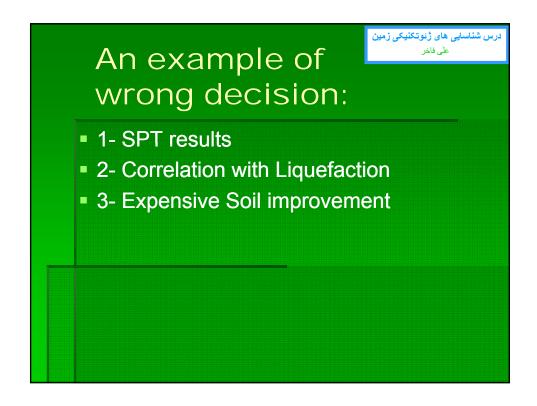
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Results of various methods of energy measurements for various hammer in Korea

Hammer Type		F2 Method		FV Method	
		Mean (%)	Standard Deviation	Mean (%)	Standard Deviation
WD	<10m	44.9	9.1	57.9	7.4
	>10m	58.9	7.3	60.2	7.8
CA	<10m	57.4	3.1	60.0	4.5
	>10m	64.9	2.6	66.4	3.4
RS	<10m	54.7	3.4	58.2	3.9
	>10m	58.0	3.1	59.0	4.1
RD	<10m	36.9	9.5	38.0	10.8
	>10m	38.6	6.7	41.4	11.7

Kim et al (2004)

# In Iran: Any important decision should not be taken based upon SPT results

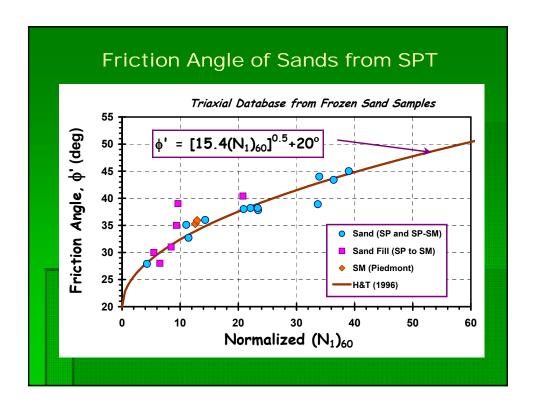


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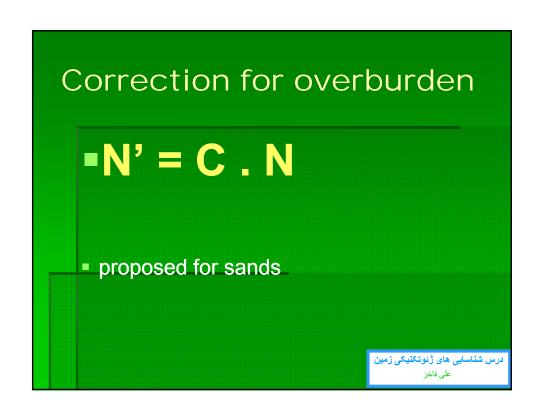
# Suggestion for important decisions about liquefaction:

A proper Site Characterization by:

CPT, and other in-situ tests improvement of sampling methods,



c<sub>u</sub> = undrained strength  $D_R$  = relative density  $\gamma_T$  = unit weight  $\gamma_T$  = unit weight  $I_R$  = rigidity index LI = liquefaction index &' = friction angle &' = friction angle OCR = overconsolidation c' = cohesion intercept K<sub>0</sub> = lateral stress state e<sub>o</sub> = void ratio e<sub>o</sub> = void ratio  $q_a$  = bearing capacity V<sub>s</sub> = shear wave  $\sigma_p'$  = preconsolidation E' = Young's modulus V<sub>s</sub> = shear wave  $C_c$  = compression index SAND E' = Young's modulus  $q_b$  = pile end bearing  $\Psi$  = dilatancy angle  $f_s$  = pile skin friction q<sub>b</sub> = pile end bearing k = permeability q<sub>a</sub> = bearing stress CLAY  $f_s$  = pile skin friction



$$(N_1)_{60} = C_N N_{60} = \left(\frac{100}{\sigma'_{vo}}\right)^{0.5} N_{60}$$

 $C_N$  = correction factor to account for the overburden pressure. As indicated in Eq. 2.5,  $C_N$  is approximately equal to  $(100/\sigma'_{vo})^{0.5}$  where  $\sigma'_{vo}$  is the vertical effective stress, in kPa.

# The effect of borehole diameter

 If the test is performed in a large diameter borehole, it is not standard.

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# Test in gravels is ambiguous

- Measurement of engineering properties of gravels is a challenge.
- The cutting shoe of the sampler may be replaced by a cone (60 deg) but it affects results.
- The use of tools of larger diameter than SPT sampler (Large Penetration Test)

# Research on the effect of grain size

## Correction for grain size

$$\frac{N_1}{D_r^2} = 9 \cdot \left(0.23 + \frac{0.06}{D_{50}}\right)^{-1.7}$$

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## Effect of Energy

- N is inversely proportional to the energy
- N should be corrected to reference value of 60% of the potential energy of the hammer.

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